

#### NEPAL ENGINEERING COUNCIL LICENSE EXAM PREPARATION COURSE

FOR

#### **CIVIL ENGINEERS**



## 4. Structural Mechanics

4.6 Indeterminate structures

### Sub topics



- Flexibility Method
- -• Slope Deflection Method
- Moment Distribution Method
- ->• Stiffness method
  - Elementary Plastic analysis

DST, DKI

#### Force Method

Forces are treated as unknown and corresponding forces are calculated.

Reducator

Degree of static indeterminacy = Unknown forces

Using compatibility conditions and force displacement relation, unknowns are found.

Consistent deformation method, Flexibility method, Moment area method, Conjugate beam method, Three moment theorem

#### Displacement Method



Displacement are treated as unknown and corresponding displacement are computed first.

Degree of kinematic indeterminacy = Unknown displacements

Using equilibrium conditions and force displacement relation, unknowns are found.

Slope deflection method, moment distribution method, stiffness method, Minimum potential energy method, Kani's method

### Flexibility and stiffness



Flexibility is displacement caused by unit force. Denoted by  $\delta$ 

Stiffness is force required for unit displacement. Denoted by k.



# Flexibility Method $\left[\Delta_{iL}\right] = \int \frac{M \times m_i}{EI} ds \qquad \left[\delta_{ij}\right] = \int \frac{m_i \times m_j}{EI} ds$ $[\Delta_{iL}] = \int \frac{P_o \times P_i}{AE} ds \qquad [\delta_{ij}] = \int \frac{P_i \times P_j}{AE} ds$



#### Flexibility Method



$$[\Delta] = [\Delta_L] + [\delta][P]$$
$$[P] = [\delta]^{-1} \{ [\Delta] - [\Delta_L] \}$$

$$\begin{bmatrix} \Delta_{iL} \end{bmatrix} = \int \frac{M \times m_i}{EI} ds \qquad \begin{bmatrix} \delta_{ij} \end{bmatrix} = \int \frac{m_i \times m_j}{EI} ds$$
$$\begin{bmatrix} \Delta_{iL} \end{bmatrix} = \int \frac{P_o \times P_i}{AE} ds \qquad \begin{bmatrix} \delta_{ij} \end{bmatrix} = \int \frac{P_i \times P_j}{AE} ds$$









# Slope Deflection Method





Point Moments









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#### Slope Deflection Method



Distributed Loads





 $\frac{5wL^2}{96}$ 



Support Movements



# Moment Distribution Method Hardy (roy Carry over factor: For a member with opposite end fixed or continuous, CO=0.5 For a member with opposite end hinged, CO=0 EI/L = Relative stiffner = 1/4 GE/C Rotational stiffness: For a member with opposite end fixed or continuous, $K = \frac{4EI}{L}$ For a member with opposite end hinged, $K = \frac{3EI}{r}$



#### Moment Distribution Method







### Moment Distribution Method

Joint	Fixed A		E	3		Pinned C
Member DF	AB 0		BA 0.516	BC 0.484		CB 1.0
FEM Balance B			-12.9	25.0 -12.1		-25.0
Carry-Over Balance C	-6.45	$\leftarrow CO = \frac{1}{2}$			$CO = 0 \rightarrow$	0 25.0
Carry-Over				12.5	$\leftarrow \text{CO} = \frac{1}{2}$	0
Balance B			-6.45	-6.05		
Carry-Over	-3.23	$\leftarrow CO = \frac{1}{2}$			$\rm CO = 0 \rightarrow$	0
SUM ∑	-9.68		-19.35	19.35		0







# 

#### $[\Delta] = [K]^{-1}\{[P] - [P']\}$



#### Stiffness method





#### Plastic analysis

plastic.

6M

5D

120KM 100KN KU



Plastic analysis is defined as the analysis in which the criterion for the design of structures is the ultimate load. The material goes beyond the elastic zone into plastic region.

 $( \mathcal{C}_{\mathcal{H}} )$  Yield stress is reached at each fiber at section irrespective of distance from neutral axis.

Analysis and design of indeterminate structure. Economic.

For purpose of analysis the strain stress curve is considered to be perfectly elasto-

#### Theorem of Plastic analysis:



• Lower bound theorem

The structure is safe and statically admissible for load W such that it is less than or equal to Collapse load ( $W_c$ ).

$$W \leq W_c$$

• Upper bound theorem

The mechanism of collapse is the one which causes the structure to fail plastically and has the lowest collapse load.

$$W \ge W_c$$



#### Methods of Plastic analysis:

• Static method

For structure with known bending moment diagram. The hinges occurs at section of highest ordinate of BMD. Based on lower bound theorem.

PANA

• Mechanism method

The plastic hinge is considered at different section for failure mechanism. Internal work done at internal hinge is equal to external work by load. Based on upper bound theorem.



### Collapse mechanism:



- Beam mechanism Plastic hinge is formed at support and/or at some point on span.
- Sway mechanism The sway occurs by formation of hinge at joints of column in frame.
- Combined mechanism Mixed form of beam and sway mechanism







Plastic Moment Capacity Ex = WS W Mp=  $W_{c} = 10M_{P}$ Mp 16 Me= Ex= Wx8  $2M_p$  $M_p$ Th = Mpx38 + Mpx38 し/、 Mp. 98 Jm = 2Mpx 28 + 3Mpx 28 = 10Mps, ( ) G.SM Mp. WL

#### Mn Terminology: 62 Plastic hinge: • It is hinge formed at a section when all the fibers at that section yield and it can no further take more moment Wc Length of plastic hinge: Mø Effected length due to plastic hinge. Point load case Mp=Wel For rectangular beam, $\frac{\iota}{3}$ point load case For I beam beam, $\frac{\iota}{6}$ • Collapse load: ( MP/My Load that makes some section undergo ultimate deformation and leads to collapse.

#### Terminology: bd g PANA ACADEM ZPX ISX621 Factor of safety: ٠ Ratio of yield moment to working load is called FOS. $FOS = \frac{M_y}{M_a}$ 6 Load factor : O • Ratio of collapse load to working load is called load factor. $\lambda = \frac{Collapse \ load}{Allowable \ load} = \frac{M_p}{M_a} = \frac{M_p}{M_a} \times \frac{V}{M_a}$ SFXFOS Plastic moment capacity: ٠ Maximum moment structure can sustain. 1 2. $M_p = \sigma_y Z_p$ $Z_p$ is plastic section modulus. $1_{i} = \sigma_{i}$



### Terminology:

$\frac{b}{b} = \frac{2h_{J}}{h}$ Shape factor: $\frac{b}{b} = \frac{2h_{J}}{h}$ Shape factor = $\frac{M_{p}}{M_{y}} = \frac{Z_{p}}{Z} = -\frac{bd^{2}/4}{bh^{2}/6} = 1.5$ (for red) $\frac{b}{b} = 2b_{J}$ $A = \frac{1}{2} \times \frac{2b}{3} \times \frac{2h}{3} = \frac{4bh}{2}$					
	Cross section	Shape Factor			
Jul, 1 24/3 24/3 Jub	Rectangular	1.5			
	Triangular	2.34			
	Solid Circular	1.7			
	Hollow Circular	1.27			
	I section	1.12-1.14			
(-6) "	T section	1.8			
	Diamond	2			





# Thank YOU !!!